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## ON METHODS FOR DETERMINING THE ROUGHNESS COEFFICIENT OF CHANNELS ALONG THE PERIMETER

**Abstract.** The scientific article deals with the issues of uniform movement of the riverbed and the determination of the roughness coefficient of the riverbed (roughness coefficient). The analysis of existing methods for calculating the difference in the roughness coefficients of channels along the perimeter, methods for determining the roughness coefficient of the slopes of channels consisting of two or three parts along the perimeter, well-known scientists - P.N.Belokon, G.K.Lotter, N.N.Pavlovsky are given. There are some varieties of calculation methods assigned for hydraulic calculations of water flow along the perimeter of a soil channel in the field of hydraulic engineering. A number of researchers states that the roughness of the channel bed imitates the motion of the flow in diverse channels with the movement of the flow under the ice layer. Nevertheless, it should be taken into account that the roughness of the channel bed has its own characteristic (specific) features of the movement of water in different open channels and below the ice cover. The common formulas proposed by number of authors for channels with different roughness along the perimeter cannot be used directly in hydraulic calculations of the flow under the ice cover, and vice versa, the equations of motion of water flow beneath the ice cover are not applied even for channels with various roughness along the perimeter. Therefore, the corresponding choice of methods for determining the roughness coefficients of the canal flow along the perimeter will be the key point to its long-term functioning.

**Keywords:** canal, uniform motion, roughness coefficient, steady motion, average speed, channel perimeter, channel cross-section, hydraulic radius.

### Introduction

Majority of canals used in hydraulic engineering have same roughness coefficient on both slopes and on bottom. Due to the long term exploitation there is a possibility of change in roughness coefficient of canal along the perimeter caused by effluent change during the vegetation period.

Changes in roughness coefficient of slopes and bottom in canal also depends on water permeability and filtration features on the basic layer. Purpose of our primary research was the common defining of roughness coefficient by perimeter of canal [1].

Small part of effluent's cross-section is key term for the appearance of canal's considered roughness coefficient from resistance along the canal's length and formation of bottom while planning flows passing through confirmed canal. Currently there are few proposed ways of calculated ratios for dealing with hydraulic accounts for water flows along the perimeter of our canal. Many researchers found similarity between flows in canals with various roughness level and flows beneath icy layer. Still, we have to emphasize the fact that they both have some unique features by themselves. Regular calculation formulas proposed by many authors for the canals with various roughness level never can be used in hydraulic accounts for flows under icy layer, as well as equations for flow beneath ice can not possibly be used for canals with different roughness indexes. Modifications made while generating this formulas can not be allowed.

### Methods and materials

Method of the research is theoretical, materials in usage was accurately processed. According to the information we have, hydraulic circumstances of flow get more complicated in cases where canal cross-

sections have different roughness because of formation of new zones, and so it leads to the fact that planar and vertical distribution of velocities along the cross-section are experiencing sharp changes. To generalize main calculating ratios we make this note - despite of the different states of canal roughness the smoothness of flow remains steadily, that is roughness of canal sides keeps constant along the all considered areas. Moreover, we accept measurements allowed by general hydraulics: magnitude of the slope which could somehow impact to the movement on different parts of flow is same, velocities on separate surfaces of any rectilinear regions of cross-section are same for the first and second parts of the flow and this equals to the maximum velocity. In general, area of canal's cross-section approximately divides to evaluate average index of canal roughness coefficient on bottom and on sides, we assume moisture perimeters  $\chi_1, \chi_2, \chi_3$  and roughness coefficient  $n_1, n_2, \dots, n_N$  of canal like that. During calculations of this well-known scientists as Horton and Einstein made this proposal [2] – velocities on considered areas are same and equal to the average speed on any point, about roughness coefficient we are defining it can be determined in this way:

$$n = \left[ \frac{\sum_{i=1}^N (\chi_i n_i^{1.5})}{\chi} \right]^{2/3} = \frac{(\chi_1 n_1^{1.5} + \chi_2 n_2^{1.5} + \dots + \chi_N n_N^{1.5})^{2/3}}{\chi^{2/3}} \quad (1)$$

Pavlovsky, Mulkhofer, Einstein and Bank [2] are recommending this equation for defining canal's roughness coefficient:

$$n = \left[ \frac{\sum_{i=1}^N (\chi_i n_i^2)}{\chi^{1/2}} \right]^{1/2} = \frac{(\chi_1 n_1^2 + \chi_2 n_2^2 + \dots + \chi_N n_N^2)^{1/2}}{\chi^{1/2}} \quad (2)$$

Another scientist Lotter proposed this way of solution [1],

$$n = \frac{\chi R^{5/3}}{\sum_{i=1}^N \left( \frac{\chi_i R_i^{5/3}}{n_i} \right)} = \frac{\chi R^{5/3}}{\frac{\chi_1 R_1^{5/3}}{n_1} + \frac{\chi_2 R_2^{5/3}}{n_2} + \dots + \frac{\chi_N R_N^{5/3}}{n_N}} \quad (3)$$

There are  $R_1, R_2, \dots, R_N$  -hydraulic radiuses of considered sectors. In the any section  $R_1 = R_2 = \dots = R_N = R$  will remain.

Canals roughness can be changed due to the ice formation on the surface of our canal. To explain this phenomenon Lotter[2] came to the conclusion that definitions of roughness for deep canals with icy surface can be determined in the following way.

Chart 1 – approximately taken measures for roughness coefficient for deep canals with icy surface

Formation of icy layer	Velocity in the canal, cm/s	Roughness coefficient
With smooth surface: Without ice floes	0,39 – 0,6 0,6-high	0,01-0,012 0,014-0,017
With ice floes	0,39-0,6 0,6-higher than	0,016-0,018 0,017-0,02
With ice floes and with rough surface	-	0,023-0,025

For instance,  $n_1$  and  $n_2$  are roughness coefficients for the canals with and without icy layer. Using first and second equations above we can find roughness coefficient for icy layer. Although coefficient defined in that way sometimes can be negative but it has no importance [1].

According to the Pavlovsky [2] to approach to the genuine solution of the equation we should accept that

the full resistance for liquid motion equals to the sum of resistances formed by canal's bottom and icy layer, so we will have

$$k v^2 \ell \chi = k_1 v^2 l \chi_1 + k_2 v^2 l \chi_2 \quad (4)$$

There 1 and 2 indexes are respectively related to the canal bottom and icy layer.

If current and roughness of canal are known, then according to Menning's formula[3] we can find the slope of flow in prismatic canal in steady motion by using given average value. Slope defined in that way is known as regular slope. Steady surface flow with regular slope could be turbulent or laminar depending on factors like flow, slope, viscosity and roughness of surface. If velocity and depth of flow are relatively low, then viscosity will be dominant factor and flow moves in laminar regime. We certainly can notice that it is not simple process from one glance to the details of flow movement with roughness coefficient.

### Results and discussions

In general finding the main calculating ratios lays down on defining moisture perimeters and areas entering to individual sections with roughness. Defining moisture perimeters of individual section is not hard at all, moreover they are determined as relevant edge of geometric figure. It should be noted that edge of rib has to lay on the considered bottom space of canal.

However, finding areas of figures adjoining to sides with individual roughness is really complicated. Methods of P.N. Belokon, G.K. Lotter, N.N. Pavlovsky, E.E. Shipenko which are known in scientific literature are directed to the significant allowances in flow's phenomenon and equality of radii of separated parts and of the whole riverbed, and in equality of average velocities on different parts and in the whole flow, but they never have been approved in practical way. Current laboratory measures made by us and other authors shows that they are significantly distinguished in max values and they have considerable mistakes in accounts. So, we came to solution that attempt of evaluating the problem of roughness coefficient by using standard methods not allowed to obtain results necessary for us: using only equation of steady motion is not enough, so according to that we have to consider another ways of dealing with this problem. We believe that dealing with equations for canals with different roughness is the most typical way which is used by all authors.

There can be negative effects caused by roughness. In practices this interaction creates following kinematics of flow's motion which can afford minimum walkthrough of considered cross-section amongst all possible conditions. In canals and open riverbeds with steady motion we can define water's velocity in this way [1, 2]:

$$v = C R^x i^y \quad (5)$$

there  $v$  – average speed, m/s;  $C$  – coefficient of Shezi;  $R$  – hydraulic radius, m;  $i$  – hydraulic slope;  $x$  and  $y$  – degree's index.

According to the famous scientist Shnekenberg[4] – main formula when canal has spiral flow has to contain following options: area of canal's cross-section, average velocity of water flow, max speed of water, moisture perimeter of canal, hydraulic radius of canal, depth, slope of flow's free surface, roughness coefficient on cross-section, effluent of major and minor loads in water, dynamic viscosity and temperature of water.

Defining roughness coefficient[2] while solving coefficient of Shezi by formulas of scientists Kutter and Menning in equation of steady motion is really hard.

### Main provisions

There was the research of rise of roughness coefficient in field conditions during 1 year in the process of determining the canal's roughness coefficient in natural conditions at university of Illinois and it's measure grew from 0.33 to 0.55 in one season. There shown the methods for defining roughness coefficient in below.

#### Method for roughness taken by all along the average canal perimeter

Meaning of this method is - measures of private parts along the canal's perimeter we write ( $n_1$  and  $n_2$ ) then appropriate to that moisture perimeters written like this  $\chi_1, \chi_2$ , knowing that we can define roughness coefficient of that type of canal by following way:

$$n_{pr} = \frac{(n_1 \chi_1 + n_2 \chi_2)}{(\chi_1 + \chi_2)} \quad (6)$$

This method is too rough. Considered roughness coefficient of canal is more dependend on hydraulic radius rather than canal's moisture perimeter.

**Method of G.K. Lotter.** G.K.Lotter [1–5] uses method of account of composite canal while calculating canals with different roughness along the canal's perimeter.

Effluent of composite canal will be:

$$Q = Q_1 + Q_2 \quad (7)$$

there:  $Q_1$  and  $Q_2$  – effluents of first and second parts of the flow.

We resolve effluent of water by formula of Shezi:

$$\omega \cdot C_{pr} \cdot \sqrt{R \cdot J} = \omega_1 \cdot C_1 \cdot \sqrt{R_1 \cdot J_1} + \omega_2 \cdot C_2 \cdot \sqrt{R_2 \cdot J_2}$$

there: –area of cross-section; –hydraulic radius of whole cross-section area; –appropriate coefficient of Shezi for whole cross-section; and –cross-section area of flow parts influenced by zones with steady roughness; and –hydraulic radiuses of first and second parts of cross-section; and –Shezi coefficient of first and second parts of cross-section;  $J$  - piezometric slope.

$\omega = R \cdot \chi$  in first and second parts of canal's cross-section and because of motion affected by slope are equal, we can write previous equation in this way:

$$C_{pr} \chi R^{3/2} = C_1 \chi_1 R_1^{3/2} + C_2 \chi_2 R_2^{3/2} \quad (8)$$

there:  $\chi$  - moisture perimeter of whole cross-section;  $\chi_1$  and  $\chi_2$  - moisture perimeter of first and second parts of cross-section.

We divide both part of given equation by  $\chi_1$  then we write  $\chi_2 / \chi_1$  proportion like (a):

$$C_{pr} = \frac{C_1 R^{3/2} + a C_2 R_2^{3/2}}{R^{3/2} (1 + a)} \quad (9)$$

From equation(9) we can see that to solve we need both - moisture perimeters of parts with different roughness and hydraulic radiuses of cross-section's personal parts. In this situation hydraulic radiuses of cross-section's personal parts are defined exactly like for composite canal. For very wide canals moisture perimeter can be equal to width of canal, about hydraulic radius - it equals to depth of considered place. In this case equation (8) will be written in the following way:

$$Q = (C_1 b_1 h_1^{3/2} + C_2 b_2 h_2^{3/2}) \sqrt{J} \quad (10)$$

There  $b_1$  and  $b_2$  - relatively width of 1 and 2 parts,  $h_1$  and  $h_2$  - depth on first and second parts.

G.K.Lotter believes that hydraulic radiuses of flow's private parts are equal to the hydraulic radius of whole flow for canals with ice formation:

$$R_1 = R_2 = R \quad (11)$$

Because of moisture perimeter of canal -  $\chi_1$  and of ice-  $\chi_2$  are equal, then hydraulic radius of whole section is equal:

$$R = \frac{\omega}{\chi_1 + \chi_2} \quad (12)$$

In this case equation(9) will be written in the following way:

$$C_{pr} = \frac{C_1 + a C_2}{(1 + a)} \quad (13)$$

Then independent on each other methods of P.N.Belokon and N.N.Pavlovsky are published[1,6].

### Method of P.N. Belokon

We consider canal's cross-section with any shape, then roughness of moisture perimeter on first part will be  $n_1$  and in the second is  $n_2$ .

We define fall along the length of canal by next formula

$$J = \frac{F}{\gamma \omega} \quad (14)$$

There:  $F$  - sum of fictitious friction forces on canal sides(walls. We mark average friction force falling on  $1\text{m}^2$  of canal's side in first part like and in second one

$$F = \tau_1 \chi_1 + \tau_2 \chi_2 \quad (15)$$

Then we can write equation(14) in following way:

$$\frac{\tau_1 \chi_1}{\gamma} + \frac{\tau_2 \chi_2}{\gamma} = \omega J$$

If take them as  $\chi_1 = a\chi$  and  $\chi_2 = a\chi$ , then we get this:

$$a_1 \frac{\tau_1}{\gamma} + a_2 \frac{\tau_2}{\gamma} = \frac{\omega}{\chi} J = RJ.$$

In turbulent steady motion it will look like this:

$$\frac{\tau_1}{\gamma} = \frac{v_1^2}{C_1^2} \quad \text{and} \quad \frac{\tau_2}{\gamma} = \frac{v_2^2}{C_2^2}$$

there:  $v_1$  and  $v_2$  - average velocity in first and second parts of flow. Then previous equations will look like that:

$$a_1 \frac{v_1^2}{C_1^2} + a_2 \frac{v_2^2}{C_2^2} = RJ \quad (16)$$

If and solved in Menning's way, then equation(16) will be like this:

$$a_1 \frac{n_1^2 v_1^2}{R_1^{1/3}} + a_2 \frac{n_2^2 v_2^2}{R_2^{1/3}} = RJ \quad (17)$$

If take this ratio like that  $\frac{n_2}{n_1} = \psi$  we will see this:

$$n_1^2 \left( a_1 \frac{v_1^2}{R_1^{1/3}} + \psi^2 a_2 \frac{v_2^2}{R_2^{1/3}} \right) = RJ \quad (18)$$

Moreover P.N.Belokon says that: if ratios for areas of cross-section relatively to zones with different roughness equals to  $\theta$ , then we get this  $\frac{\omega_1}{\omega_2} = \theta$

Then  $\omega_1 + \omega_2 = \omega$  will be when  $\omega_1 = \frac{\theta}{1+\theta} \omega$  and  $\omega_2 = \frac{\theta}{1+\theta} \omega$ .

P.N.Belokon finds answers of  $R_1$  and  $R_2$  from next equations:

$$R_1 = \frac{\omega_1}{\chi_1} = \frac{\theta}{a_1(1+\theta)} R \quad \text{and} \quad R_2 = \frac{\omega_2}{\chi_2} = \frac{\theta}{a_2(1+\theta)} R \quad (19)$$

Full effluent will be:  $Q_1 + Q_2 = Q$ .

We resolve effluent by Shezi-Menning formula:

$$Q_1 = \frac{1}{n_1} \omega_1 R_1^{2/3} J^{1/2} \text{ and } Q_2 = \frac{1}{n_2} \omega_2 R_2^{2/3} J^{1/2} \quad (20)$$

We divide first part of equation(20) by second part because the slopes of them are equal: or we put answers of  $\omega_1, \omega_2$  and  $R_1, R_2$  into this  $\frac{Q_1}{Q_2} = \frac{n_1}{n_2} \cdot \frac{R_1^{2/3} \omega_1}{R_2^{2/3} \omega_2}$  equation:

$$\frac{Q_1}{Q_2} = \psi \left( \frac{a_2}{a_1} \right)^{2/3} \theta^{2/3}$$

If we convert that equations, roughness coefficient will be:

$$n = n_1 (a_1 + a_2 \psi^{3/2})^{3/2} \quad (21)$$

### Method of N.N. Pavlovsky

According to N.N.Pavlovsky [1,7–20] definition of considered roughness coefficient is taken from equation of steady motion for open canals written below:

$$RJ = \frac{\tau}{\gamma} \quad (22)$$

If sides of canal are distinguished from each other then we have to exchange this equation with another one, for that we need to divide liquid between cross-sections, direct impacting forces to direction of flow, then construct new motion equation for released part.

Then we get this equation(22):

$$RJ = \frac{\tau_1 + a\tau_2}{\gamma(1+a)} \quad (23)$$

there:and-respectively average special friction in sides of canal, – ratio of moisture perimeters on two sides -  $a = \frac{\chi_2}{\chi_1}$ .

When sides are same average special friction in sides of canal is equal to the average depth in this subsequence  $\frac{\tau}{\gamma} = \frac{v^2}{C^2}$ .

According to that we can write and like this:

$$\frac{\tau_1}{\gamma} = \frac{v^2}{C^2}, \quad \frac{\tau_2}{\gamma} = \frac{v^2}{C^2} \quad (24)$$

Before getting binders N.N.Pavlovsky takes that the average velocity of each part is equal -  $v = v_1 = v_2$ . We put this equation(24) to another one(23):

$$RJ = \frac{v^2}{1+a} \left( \frac{1}{C_1^2} + \frac{a}{C_2^2} \right), \text{ follows to } v = C_1 C_2 \sqrt{\frac{1+a}{aC_1^2 + C_2^2}} \cdot \sqrt{RJ} \quad (25)$$

N.N.Pavlovsky counts measure before  $\sqrt{RJ}$  as “coefficient of Shezi”, then marks it like  $C_{pr}$ , because of that we can write equation(25) in next way:

$$v = C_{pr} \cdot \sqrt{RJ} \quad (26)$$

Here  $C_{pr} = C_1 C_2 \sqrt{\frac{1+a}{aC_1^2 + C_2^2}}$ .

To find  $C_1$  and  $C_2$  we have to know measures of  $\omega$  and  $\chi$ . To determine them Pavlovsky proposed to count areas of private cross-section parts of canal and moisture perimeters as proportional, then

$$\frac{\omega_1}{\omega_2} = \frac{\chi_1}{\chi_2}, \text{ after that } \frac{\omega_1}{\omega} = \frac{\chi_1}{\chi}, \frac{\omega_2}{\omega} = \frac{\chi_2}{\chi} \quad (27)$$

Then we get:

$$R_1 = \frac{\omega_1}{\chi_1} = \frac{\omega}{\chi} = R, R_2 = \frac{\omega_2}{\chi_2} = \frac{\omega}{\chi} = R \quad (28)$$

$$\text{Then, } R_1 = R_2 = R \quad (29)$$

We can get next equation while solving coefficient of Shezi, of course by knowing measure of hydraulic radius, using this formula and define necessary equation for considered roughness coefficient:

$$C_{pr} = R^y \sqrt{\frac{1+a}{n_1^2 + a n_2^2}} \quad (30)$$

Applying information about considered roughness coefficient, Pavlovsky reforms equation (30) into this sight:

$$\text{there: } n = \sqrt{\frac{n_1^2 + a \cdot n_2^2}{1+a}} \quad (31)$$

If sides of canal are from three different parts and relatively moisture perimeters of three parts are not same, then equation for considered roughness coefficient will be like this:

$$n_{pr} = \sqrt{\frac{n_1^2 + a \cdot n_2^2 + a \cdot n_3^2}{1+a}}, \quad (32)$$

$$\text{there: } a^1 = \frac{\chi_3}{\chi_1}.$$

By concluding, N.N. Pavlovsky writes this - method for defining roughness coefficient might be changed from anticipated extra researches in future [1,2,15-23].

### Conclusion

Many factors influent to roughness coefficient of canal's bottom in field conditions, by considering them, famous scientist Covan proposed this way to deal with considered roughness coefficient [2]:

$$n = (n_0 + n_1 + n_2 + n_3 + n_4) m_5 \quad (33)$$

there,  $n_0$  – measure of roughness coefficient for canal's bottom with natural, smooth soil;  $n_1$  – coefficient for bottoms with different soil;  $n_2$  – coefficient for measuring and finding canal's cross-section;  $n_3$  – coefficient for measuring possibility of barriers in the canal;  $n_4$  – coefficient for describing regime of flow in the canal and for impact of resistance caused by plants outgrowth;  $m_5$  – coefficient for measuring aftereffect of winding for considered water canal.

According to results of calculations, we get formulated measure of  $n$ :

$$n = \frac{(x-1)h^{1/6}}{6,78(x+0,95)} \quad (34)$$



By concluding equation directed on determining of roughness coefficient we get this (34) equation. To approve reality of this equation were taken experimental results of works on connections between canal roughness and average depth. Analyze of data taken from results of research shows that there is a some connection between coefficient of Menning and distribution of velocities along the canal's cross-section. To find out how much taken equation can be applied on manufacture, there would be needed data taken from researches occurred in laboratory and natural conditions [1–8, 23–30].

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**ПЕРИМЕТРІ БОЙЫНША КАНАЛДАРДЫҢ КЕДІР-БҰДЫРЛЫҚ  
КОЭФФИЦИЕНТІН АНЫҚТАУ ӘДІСТЕРІ ТУРАЛЫ**

**Андатпа.** Ғылыми мақалада арнаның бірқалыпты қозғалысын және арнаның кедір-бұдырлық коэффициентін (кедір-бұдырлық коэффициенті) анықтау мәселелері қарастырылады. П.Н. Белоконь, Г.К. Лотер, Н.Н. Павловский сияқты атақты ғалымдардың периметрі бойынша каналдардың екі-үш бөлігінен тұратын кедір-бұдырлық коэффициентін анықтау әдістеріне, периметрі бойынша арналардың кедір-бұдырлық коэффициенттерінің айырмашылығын есептеудің қолданыстағы әдістеріне талдау жасалды. Гидротехника саласында жер асты каналының периметрі бойынша су ағынының гидравликалық есептеулері үшін ұсынылатын бірқатар есептеу әдістері бар. Бірқатар зерттеушілер арнаның кедір-бұдырлығы мұз қабатының астындағы ағынның қозғалысымен әртүрлі арналардағы ағынның қозғалысына ұқсайды деп мәлімдейді. Бірақ канал арнасының кедір-бұдырлығы әртүрлі ашық арналарда және мұз қабаты астындағы су қозғалысының өзіне тән (спецификалық) ерекшеліктеріне ие екенін ескеру қажет. Көптеген авторлардың ұсынған периметрі бойынша әр түрлі кедір-бұдырлы арналарды есептеу формулаларын мұз қабатының астындағы гидравликалық ағынды есептеулерде тікелей қолдану мүмкін емес, тіпті керісінше мұз қабатының астындағы су ағынының қозғалыс теңдеулерінің периметрі әр түрлі кедір-бұдырлы арналарды есептеуде де қолданылмайды. Сол себепті табан периметрі бойынша су арналарының кедір-бұдырлық коэффициенттерін анықтау әдістерін дұрыс таңдау, оның ұзақ мерзімді жұмыс істеуінің кепілі болады.

**Тірек сөздер:** арна, бірқалыпты қозғалыс, кедір-бұдырлық коэффициенті, тұрақты қозғалыс, орташа жылдамдық, арна периметрі, арнаның өтім (көлденен) қимасы (поперечное сечение), гидравликалық радиус.

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**О МЕТОДАХ ОПРЕДЕЛЕНИЯ КОЭФФИЦИЕНТА ШЕРОХОВАТОСТИ КАНАЛОВ  
ПО ПЕРИМЕТРУ**

**Аннотация.** В научной статье рассматриваются вопросы равномерного движения русла и определения коэффициента шероховатости русла (коэффициента шероховатости). Приводится анализ существующих методов расчета разности коэффициентов шероховатости каналов по периметру, методы определения коэффициента шероховатости, состоящих из двух-трех частей откосов каналов по периметру, таких известных ученых, как П.Н. Белоконь, Г.К. Лоттер, Н.Н. Павловских. В области гидротехники существует ряд расчетных методов, предлагаемых для гидравлических расчетов расхода воды по периметру грунтового канала. Ряд исследователей утверждают, что шероховатость русла канала имитирует движение потока в разных каналах с движением потока под слоем льда. Но следует учитывать, что шероховатость русла канала имеет свои характерные (специфические) особенности движения воды в различных открытых каналах и под ледяным покровом. Расчетные формулы,

предложенные многими авторами для каналов с разной шероховатостью по периметру, не могут быть использованы непосредственно при гидравлических расчетах потока под ледяным покровом, и, наоборот, уравнения движения потока воды под ледяным покровом не применяются даже для каналов с разной шероховатостью по периметру. Поэтому правильный выбор методов определения коэффициентов шероховатости русла каналов по периметру будет залогом его длительного функционирования.

**Ключевые слова:** канал, равномерное движение, коэффициент шероховатости, установившееся движение, средняя скорость, периметр канала, поперечное сечение канала, гидравлический радиус.